

Land Dynamics Australia

Geotechnical Site Classification

Proposed Residential Subdivision

Stage 1 The Sanctuary, 344 John Oxley Drive, Thrumster

Report No. RGS21087.1-AP_Rev1

21 June 2025



RGS21087.1-AP_Rev1

21 June 2025

Land Dynamics Australia
77 Lord Street
PORT MACQUARIE NSW 2444

Attention: Jodie Chapman

Dear Jodie,

**RE: Proposed Residential Subdivision – Stage 1 The Sanctuary, 344 John Oxley Drive,
Thrumster**

Geotechnical Site Classification

As requested, Regional Geotechnical Solutions Pty Ltd (RGS) has undertaken a geotechnical site classification in accordance with AS2870-2011 *Residential Slabs and Footings* for the proposed residential lots located in Stage 1 of The Sanctuary Estate, 344 John Oxley Drive (Lot 1 DP 1245588) Thrumster. Stage 1 comprises Lot No's 101 to 136 as shown on the supplied plan titled "Plan of Subdivision of Lot 1 DP 12455888".

Based on the existing profiles encountered at the time of the field investigations and on the basis that all fill present in the fill platform was placed under Level One Inspection and Testing as defined in AS3798-2007, the building areas within the lots present are classified in accordance with AS2870-2011 *Residential Slabs and Footings* as detailed in the attached report.

If you have any questions regarding this project, please contact the undersigned.

For and on behalf of **Regional Geotechnical Solutions Pty Ltd**

Prepared by



Grant Colliar

Senior Engineering Geologist

Reviewed by



Steven Morton

Principal Geotechnical Engineer



Table of Contents

1	INTRODUCTION	1
2	METHODOLOGY	1
3	LABORATORY TESTING	1
4	SITE CONDITIONS	2
4.1	Surface Conditions	2
4.2	Subsurface Conditions	3
5	SITE CLASSIFICATION	5
6	CONSTRUCTION AND SITE MAINTENANCE CONSIDERATIONS	6
7	LIMITATIONS.....	7

Figures

Figure 1 Investigation Location Plan

Appendices

Appendix A Results of Field Investigations

Appendix B Laboratory Test Result Sheets



1 INTRODUCTION

Regional Geotechnical Solutions Pty Ltd (RGS) has undertaken a geotechnical site classification in accordance with AS2870-2011 *Residential Slabs and Footings* for the proposed residential lots located in Stage 1 of The Sanctuary Estate, 344 John Oxley Drive (Lot 1 DP 1245588) Thrumster. Stage 1 comprises Lot No's 101 to 136 as shown on the supplied plan titled "Plan of Subdivision of Lot 1 DP 1245588".

The majority of the residential lots have been modified by site regrading works comprising up to approximately 5m cut, or placement of up to approximately 3m of clay fill. Filling works was undertaken by Kazac Civil Pty Ltd, with Level One Inspection and Testing of the works undertaken by Douglas CMG Pty Ltd. A copy of the Level One report has been provided to RGS.

2 METHODOLOGY

Field work for the assessment was undertaken on 20 December 2023 and was based on the supplied drawing titled "Plan of Subdivision of Lot 1 DP 1245588". Fieldwork was undertaken by an Engineering Geologist from RGS and included:

- Observation of site features and surrounding features relevant to the geotechnical conditions of the site;
- 19 boreholes undertaken by a 4WD mounted drilling rig to depths of between 1.5m and 2m logged and sampled by an Engineering Geologist; and
- U50 tube samples collected from soil horizons considered representative of cohesive soil profiles.

Engineering logs of the boreholes are presented in Appendix A. Investigation locations are shown on Figure 1 and were obtained by measurements to prominent site features. Coordinates of investigation locations were recorded using a hand held GPS and the coordinates are shown on the engineering logs. Reduced levels at the borehole locations were estimated from the supplied drawings and are shown on the engineering logs.

3 LABORATORY TESTING

Samples considered representative of foundation soils were submitted to a NATA accredited laboratory for the measurement of soil volume change over an extreme range of moisture content (shrink / swell index) on nine U50 samples. Atterberg Limits testing was undertaken on one sample that crumbled during extraction from the U50 tube. Results are presented in Appendix B and summarised in Table 1.

Table 1: Laboratory Testing Summary

Location	Depth (m)	Lot	Material	Shrink Swell Index (%)	Linear Shrinkage (%)	Plasticity Index (%)
BH202	0.4 – 0.7	105	Residual Clay	1.8	-	-
BH203	0.3 – 0.7	110	Fill Clay	2.3	-	-
BH205	0.4 – 0.7	136	Residual Clay	1.1	-	-



Location	Depth (m)	Lot	Material	Shrink Swell Index (%)	Linear Shrinkage (%)	Plasticity Index (%)
BH208	0.7 – 1.1	116/117	EW Slate	1.0	-	-
BH210	0.3 – 0.7	120/121	EW Slate	1.4	-	-
BH212	0.3 – 0.7	124/125	Residual Clay	1.8	-	-
BH214	0.3 – 0.7	128/129	EW Dolerite	1.0	-	-
BH217	0.3 – 0.7	108/109	Fill Clay	0.6	-	-
BH218*	0.3 – 0.6	101/102	Fill Clay	-	5.0	13.0

Note: * Atterberg Limits testing undertaken on U50 sample that crumbled upon extraction

4 SITE CONDITIONS

4.1 Surface Conditions

Stage 1 is located to the north of John Oxley Drive in an area of gently to moderately undulating topography where it is situated on the north east and east facing slopes a low hill that is up to RL 22m in elevation. Surface elevations across the site range from approximately RL 22m along the western boundary to approximately RL 8.5m along the eastern boundary.

A satellite image of the site that shows the location of the site and the site setting is reproduced below.



Satellite image dated 2023 obtained from Google Earth that illustrates the site location and setting. The approximate site boundaries for Stage 1 are outlined in red.



Surface slopes have been modified by cut and fill and range from approximately 2° – 8°. Some lots have been terraced and are separated by retaining walls that are up to approximately 2m height.

Lots 101 – 112 and 136 have been modified by earthworks comprising placement of more than 0.4m of clay fill that was placed under Level One inspection and monitoring as defined in AS3798-2007 *Guidelines on Earthworks for Commercial and Residential Developments* by Douglas Partners refer Level 1 Geotechnical Certification and Report, by Douglas - Project 209310, dated 10 November 2022. The approximate extent of the fill areas is shown on Figure 1.

Drainage of the site would be via a combination of overland flow and surface infiltration.

Selected site images are presented below.



4.2 Subsurface Conditions

The site is situated in an area underlain by deeply weathered geological units of the Port Macquarie Block which includes weathered slate and dolerite.

The materials encountered during the investigation are summarised in Table 2 and 3. Further details are presented on the engineering logs in Appendix B.

Table 2: Summary of Geotechnical Units

Unit	Material	Material Description
UNIT 1A	TOPSOIL/ FILL	Sandy SILT to Sandy CLAY, low plasticity, dark brown/brown, traces of gravel
UNIT 1B	FILL – CLAY (CONTROLLED)	Sandy Silty CLAY to Sandy CLAY to Silty CLAY, medium to high plasticity, pale brown/pale red/pale grey, very stiff, traces to some gravel, fine to medium



Unit	Material	Material Description
UNIT 2	RESIDUAL	Sandy CLAY to Sandy Silty CLAY, medium to high plasticity, pale red/red/brown, very stiff, traces of gravel, fine
UNIT 3	EW SLATE	Extremely Weathered SLATE, recovered as Silty CLAY or Sandy Silty CLAY, medium to high plasticity, pale red/pale grey/grey/pale red, traces of rock fabric
UNIT 4	EW DOLERITE	Extremely Weathered dolerite, recovered as Sandy CLAY, medium to high plasticity, yellow/pale brown, traces or rock fabric
UNIT 5	HW TO MW SLATE	Highly to Moderately Weathered Slate, pale grey/white/pale orange, inferred very low to low strength, foliated, recovered as Silty SAND to Sandy SILT

Table 3: Summary of Subsurface Profiles - Depth to Base of Material Layer (m)

BH	Lot	Unit 1A – Fill/ Topsoil	Unit 1B - Controlled Fill Clay	Unit 2 - Residual	Unit 3 - EW Slate	Unit 4 - EW Dolerite	Unit 5 - HW to MW Slate
BH201	103	0.15	0.7	≥ 1.5	-	-	-
BH202	105	0.2	1.0	-	≥ 1.5	-	-
BH203	110	0.15	0.5	≥ 1.5	-	-	-
BH204	111/112	0.15	1.5	≥2.0	-	-	-
BH205	136	0.2	0.4	1.3	≥ 1.5	-	-
BH206	113/134	0.1	--	-	≥ 1.5	-	-
BH207	114/115	0.2	-	-	-	-	≥ 1.5
BH208	116/117	0.15	--	-	≥ 1.5	-	-
BH209	118/119	0.15	-	-	-	-	≥ 1.5
BH210	120/121	0.1	-	-	≥ 1.5	-	-
BH211	122/123	0.15	-	-	0.5	-	≥ 1.5
BH212	124/125	0.1	-	≥ 1.5	-	-	-
BH213	126/127	0.1	-	≥ 1.5	-	-	-
BH214	128/129	0.15	-	-	-	≥ 1.5	-
BH215	130/131	0.15	-	-	-	≥ 1.5	-
BH216	132/133	0.2	-	-	-	-	≥ 1.5
BH217	108/109	0.15	≥ 1.5	-	-	-	-



BH	Lot	Unit 1A – Fill/ Topsoil	Unit 1B - Controlled Fill Clay	Unit 2 - Residual	Unit 3 - EW Slate	Unit 4 - EW Dolerite	Unit 5 - HW to MW Slate
BH218	101/102	0.15	0.7	-	-	-	≥ 1.5
BH219	106/107	0.15	-	≥ 1.5	-	-	-

Note: ≥ Indicates that base of material layer was not encountered
 -- Indicates that the material was not encountered at the test location

Groundwater was not encountered within the boreholes. It should be noted that fluctuations in groundwater levels can occur as a result of seasonal variations, temperature, rainfall and other similar factors, the influence of which may not have been apparent at the time of the assessment.

5 SITE CLASSIFICATION

For structures or components that are similar in construction, performance expectation, and loading to a typical domestic structure, the guidance provided in AS2870-2011 "*Residential Slabs and Footings*" would be appropriate.

In assessing the estimated characteristic surface movement (y_s) values the following has been adopted:

- All clay fill of > 0.4m thickness was placed under Level 1 Inspection and Testing as defined in AS3798-2007, and can therefore be considered as Controlled Fill;
- Where there was cut undertaken the depth of cracked zone was reduced by the depth of cut;
- Suction change at ground surface of pf 1.2;
- Depth of suction change of 1.5m;
- Characteristic I_{ss} for controlled clay fill of between 0.6 and 2.3%;
- Characteristic I_{ss} for residual clay of between 1.1 and 1.8%;
- Characteristic I_{ss} for extremely weathered slate of between 1.0 and 1.4%; and
- Characteristic I_{ss} for extremely weathered dolerite of 1.0%.

The proposed building areas for Lot No's 101 – 112 and 136 have been modified by filling works of >0.4m thickness. These lots are classified as Class P in accordance with AS2870-2011, Clause 2.5.3 Section (a) due to the presence of clay fill of >0.4m, requiring footings to be designed in accordance with engineering principles.

The building areas for the lots modified by filling works undertaken under Level One supervision have been reclassified as summarised in Table 4 in accordance with Clause 2.5.3 Section (c) of AS2870-2011, based on the existing profiles at the time of field investigation, the properties of the Controlled Fill that was placed under Level One supervision as defined by AS3798- 2007, the properties of the underlying natural profile, and the estimated surface movement (y_s).



Table 4: Site Classification Summary

Lots	Site Re-classification	Expected Surface Movement $y_s + y_t$ (mm)
114, 115, 118, 119, 122, 123, 132, 133	Class S	20mm
101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 116, 117, 120, 121, 124, 125, 126, 127, 128, 129, 130, 131, 134, 135, 136	Class M	30 – 40mm

6 CONSTRUCTION AND SITE MAINTENANCE CONSIDERATIONS

All structural footings should be founded as follows:

- All footings should be founded in Controlled Fill or natural soils below all topsoil and uncontrolled fill materials;
- Footings can be designed on the basis of a maximum allowable base bearing pressure of **100kPa** for footings founded within the Controlled Fill, residual clay or extremely weathered slate of at least very stiff strength. Footings founded in highly to moderately weathered rock of at least low strength can be designed on the basis of a maximum allowable base bearing pressure of **300kPa**;
- All footings, edge beams and internal beams should be entirely founded on similar material and outside or below the zones of influence resulting from existing or future service trenches and other subsurface structures;
- The engineering design for the retaining walls present allows for any surcharge affecting the walls such as proposed footings, structures or sloping surfaces;
- The soils in the Port Macquarie area are prone to fretting and softening on exposure to air and water. It is therefore recommended that concrete be poured as soon as possible after footing excavation. In the event that wet weather occurs prior to pouring of concrete, the base of footing excavations should be checked for the presence of loose or softened material, which should be removed prior to pouring concrete; and
- Prior to the placement of concrete we recommend that footings be observed and assessed by a suitably experienced geotechnical engineer to assess that the correct founding material has been achieved.

Where further lot filling works are proposed, all fill for the support of structures should be placed and compacted in accordance with the recommendations outlined in AS3798-2007 *Guidelines on Earthworks for Residential and Commercial Developments*, under Level 1 supervision, for it to be considered Controlled Fill as defined in AS2870-2011. The founding of structures on fill that is not placed in accordance with Level 1 requirements is not recommended.

Site maintenance must comply with the recommendations and advice provided in CSIRO Sheet BTF18 "*Foundation Maintenance and Footing Performance: A Homeowners Guide*" a copy of which is available from the CSIRO website <http://www.publish.csiro.au/pid/7076.htm>

Shrink-swell related movements can be affected by alterations to the soil profile by cutting and filling, and by the suction related effects of trees close to the building area. The effects of any such cutting, filling, tree planting should be taken into account when selecting design values for differential movement across the building.



7 LIMITATIONS

This report comprises the results of an investigation carried out for a specific purpose and client as defined in the document. The report should not be used by other parties or for purposes or projects other than those assumed and stated within the report, as it may not contain adequate or appropriate information for applications other than those assumed or advised at the time of its preparation. The contents of the report are for the sole use of the client and no responsibility or liability will be accepted to any third party. The report should not be reproduced either in part or in full, without the express permission of Regional Geotechnical Solutions Pty Ltd.

Geotechnical site investigation is based on data collection, judgment, experience, and opinion. By its nature, it is less exact than other engineering disciplines. The findings presented in this report and used as the basis for the recommendations presented herein were obtained using normal, industry accepted geotechnical design practises and standards. To our knowledge, they represent a reasonable interpretation of the general condition of the site. Under no circumstances, however, can it be considered that these findings represent the actual state of the site at all points.

The recommended depth and properties of any soil, rock, groundwater, or other material referred to in this report is an engineering estimate based on the information available at the time of its writing. The estimate is influenced and limited by the fieldwork method and testing carried out in the site investigation, and other relevant information as has been made available. In cases where information has been provided to Regional Geotechnical Solutions for the purposes of preparing this report it has been assumed that the information is accurate and appropriate for such use. No responsibility is accepted by Regional Geotechnical Solutions for inaccuracies within any data supplied by others.

If site conditions encountered during construction vary significantly from those discussed in this report, Regional Geotechnical Solutions Pty Ltd should be contacted for further advice.

This report alone should not be used by contractors as the basis for preparation of tender documents or project estimates. Contractors using this report as a basis for preparation of tender documents should avail themselves of all relevant background information regarding the site before deciding on selection of construction materials and equipment.

If you have any questions regarding this project, or require any additional consultations, please contact the undersigned.

For and on behalf of **Regional Geotechnical Solutions Pty Ltd**

Prepared by

Grant Colliar

Senior Engineering Geologist

Reviewed by

Steven Morton

Principal Geotechnical Engineer



Figures



Appendix A

Results of Field Investigations



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH201**

CLIENT: Land Dynamics Australia

PAGE: 1 of 1

PROJECT NAME: Stage 1, The Sanctuary

JOB NO: RGS21087.1

SITE LOCATION: John Oxley Drive, Port Macquarie

LOGGED BY: GC

TEST LOCATION: Lot 103

DATE: 20/12/23

DRILL TYPE: RGS Ute Mounted Drill Rig

EASTING: 485517 m

SURFACE RL: 9.8 m

BOREHOLE DIAMETER: 120 mm

INCLINATION: 90°

NORTHING: 6519705 m

DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered		9.5	0.5		ML	FILL: Sandy SILT, brown, sand, fine to medium grained	D				FILL/TOPSOIL
						CI	FILL: Silty CLAY, medium plasticity, pale brown with pale grey mottling	M < w _p	Fr / VSt	HP	250	FILL-CLAY
										HP	300	
						CI	Sandy CLAY: Medium plasticity, red, traces of pale brown			HP	300	RESIDUAL
				1.0								
				0.5								
				9.0								
				8.5								
				1.5								
				8.0								
				2.0								
				7.5								
				2.5								
				7.0								
							Hole Terminated at 1.50 m					

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density		V	Very Loose	Density Index <15%
		HP Hand Penetrometer test (UCS kPa)		L	Loose	MD	Medium Dense	Density Index 15 - 35%
				D	Dense			Density Index 35 - 65%
				VD	Very Dense			Density Index 65 - 85%
								Density Index 85 - 100%

RG 2.00.3 LIB GLB Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH200 SERIES LOGS.GPJ <DrawingFile> 30/1/2024 11:04 10.03.00.08 Dargal Lab and In Situ Tool - DGD Lib RG 2.00.3 2022-03-03 Pjt RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH202**
PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

CLIENT: Land Dynamics Australia
PROJECT NAME: Stage 1, The Sanctuary
SITE LOCATION: John Oxley Drive, Port Macquarie
TEST LOCATION: Lot 105

DRILL TYPE: RGS Ute Mounted Drill Rig
BOREHOLE DIAMETER: 120 mm
INCLINATION: 90°
EASTING: 485477 m
NORTHING: 6519693 m
SURFACE RL: 11.0 m
DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered					ML	FILL: Sandy SILT, dark brown, sand, fine to medium grained	D				FILL/TOPSOIL
		0.40m				CH	Sandy Silty CLAY: Medium to high plasticity, pale red with pale brown mottling, traces of gravel, fine grained, rounded	M < w _p	Fr / VSt	HP	250	RESIDUAL
		U50 0.70m	10.5	0.5		CH	Silty CLAY: Medium to high plasticity, pale red with pale grey/white mottling, traces of rock fabric	M > w _p	St	HP	150	EXTREMELY WEATHERED SLATE
			10.0	1.0		CH						
			9.5	1.5			Hole Terminated at 1.50 m					
			9.0	2.0								
			8.5	2.5								

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	w _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	w _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density	V	Very Loose	Density Index <15%	
		HP Hand Penetrometer test (UCS kPa)			L	Loose	Density Index 15 - 35%	
					MD	Medium Dense	Density Index 35 - 65%	
					D	Dense	Density Index 65 - 85%	
					VD	Very Dense	Density Index 85 - 100%	

RG 2.00.3 LIB GLB Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH200 SERIES LOGS.GPJ <<DrawingFile>> 30/1/2024 11:04 10.03.00.09 Dargal Lab and In Situ Tool - DGD Lib RG 2.00.3 2022-03-03 PJT RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH203**

CLIENT: Land Dynamics Australia
PROJECT NAME: Stage 1, The Sanctuary
SITE LOCATION: John Oxley Drive, Port Macquarie
TEST LOCATION: Lot 110

PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

DRILL TYPE: RGS Ute Mounted Drill Rig
BOREHOLE DIAMETER: 120 mm
INCLINATION: 90°
EASTING: 485500 m
NORTHING: 6519750 m
SURFACE RL: 9.3 m
DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered	0.30m	<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div>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LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density		V	Very Loose	Density Index <15%
		HP Hand Penetrometer test (UCS kPa)		L	Loose			Density Index 15 - 35%
				MD	Medium Dense			Density Index 35 - 65%
				D	Dense			Density Index 65 - 85%
				VD	Very Dense			Density Index 85 - 100%

RG 2.00.3 LIB G.LB Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH200 SERIES LOGS.GPJ <DrawingFile> 30/1/2024 11:04 10.03.00.09 Dargal Lab and in Situ Tool - DGD Lib RG 2.00.3 2022-03-03 Pjt RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH204**CLIENT:** Land Dynamics Australia**PAGE:** 1 of 1**PROJECT NAME:** Stage 1, The Sanctuary**JOB NO:** RGS21087.1**SITE LOCATION:** John Oxley Drive, Port Macquarie**LOGGED BY:** GC**TEST LOCATION:** Lot 111/112**DATE:** 20/12/23**DRILL TYPE:** RGS Ute Mounted Drill Rig**EASTING:** 485467 m**SURFACE RL:** 10.8 m**BOREHOLE DIAMETER:** 120 mm**INCLINATION:** 90°**NORTHING:** 6519751 m**DATUM:** AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered		10.5	0.5		ML	FILL: Sandy SILT, dark brown, traces of gravel, fine grained, subangular	D		HP	250	FILL/TOPSOIL
						CI	FILL: Silty CLAY, medium plasticity, pale grey/pale brown, traces of gravel, fine to medium grained, subangular	M < w _p	Fr / VSt			FILL-CLAY
						CH	FILL: Sandy CLAY, medium to high plasticity, brown, traces of white/yellow sand, fine to medium grained, traces of gravel, fine grained, subangular					
						CI	FILL: Silty CLAY, medium plasticity, pale red/pale brown with white mottling					
						CH	Sandy CLAY: Medium plasticity, red, sand, fine to medium grained					RESIDUAL
			8.5	2.5			Hole Terminated at 2.00 m					
			8.0									

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density	V	Very Loose	Density Index <15%	
		HP Hand Penetrometer test (UCS kPa)			L	Loose	Density Index 15 - 35%	
					MD	Medium Dense	Density Index 35 - 65%	
					D	Dense	Density Index 65 - 85%	
					VD	Very Dense	Density Index 85 - 100%	

RG 2.00.3.LIB.GLB.Log_RG_NON-CORED BOREHOLE - TEST PIT_RGS21087.1 BH200 SERIES LOGS.GPJ <DrawingFile> 30/1/2024 11:04 10.03.00.09 Dargal Lab and in Situ Tool - DGD.LIB.RG 2.00.3 2022-03-03 PJT_RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH205**
PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

CLIENT: Land Dynamics Australia
PROJECT NAME: Stage 1, The Sanctuary
SITE LOCATION: John Oxley Drive, Port Macquarie
TEST LOCATION: Lot 136

DRILL TYPE: RGS Ute Mounted Drill Rig
BOREHOLE DIAMETER: 120 mm
INCLINATION: 90°
EASTING: 485445 m
NORTHING: 6519775 m
SURFACE RL: 13.0 m
DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered					ML	FILL: Sandy SILT, dark brown, traces of gravel, fine to medium grained, subrounded	D				FILL/TOPSOIL
		0.40m				CI	FILL: Silty CLAY, medium plasticity, pale brown/pale red	M < W _p	Fr / VSt	HP	250	FILL-CLAY
		U50	12.5	0.5		CH	Sandy CLAY: Medium to high plasticity, red, sand, fine to medium grained				390	RESIDUAL
											400	
		0.70m									390	
			12.0	1.0								
						CI	Silty CLAY: Medium plasticity, pale grey/white with red mottling, traces of rock fabric			HP	370	EXTREMELY WEATHERED SLATE
			11.5	1.5								
							Hole Terminated at 1.50 m					
			11.0	2.0								
			10.5	2.5								

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density		V	Very Loose	Density Index <15%
		HP Hand Penetrometer test (UCS kPa)		L	Loose			Density Index 15 - 35%
				MD	Medium Dense			Density Index 35 - 65%
				D	Dense			Density Index 65 - 85%
				VD	Very Dense			Density Index 85 - 100%

RG 2.00.3 LIB GLB Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH200 SERIES LOGS.GPJ <<DrawingFile>> 30/1/2024 11:04 10.03.00.09 Dargal Lab and in Situ Tool - DGD Lib RG 2.00.3 2022-03-03 PJT RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH206**
PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

CLIENT: Land Dynamics Australia
PROJECT NAME: Stage 1, The Sanctuary
SITE LOCATION: John Oxley Drive, Port Macquarie
TEST LOCATION: Lot 113/134

DRILL TYPE: RGS Ute Mounted Drill Rig
BOREHOLE DIAMETER: 120 mm
EASTING: 485425 m
NORTHING: 6519739 m
SURFACE RL: 13.5 m
DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered					ML	FILL: Sandy SILT, dark brown, traces of gravel, fine grained, subangular, sand, fine to medium grained	D	Fr	HP	250	FILL/TOPSOIL
						CH	Sandy Silty CLAY: Medium to high plasticity, white/pale grey with red mottling, traces of rock fabric	M < w _p	Fr / VSt			HP
							Hole Terminated at 1.50 m					
											</	

RG 2.00.3.LIB.GLB.Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH206 SERIES LOGS.GPJ <<DrawingFile>> 30/1/2024 11:04 10.03.00.09 Dargal Lab and in Situ Tool - DGD Lib RG 2.00.3 2022-03-03 PJT.RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH207**
PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

CLIENT: Land Dynamics Australia
PROJECT NAME: Stage 1, The Sanctuary
SITE LOCATION: John Oxley Drive, Port Macquarie
TEST LOCATION: Lot 114/115

DRILL TYPE: RGS Ute Mounted Drill Rig
BOREHOLE DIAMETER: 120 mm
INCLINATION: 90°
EASTING: 485405 m
NORTHING: 6519709 m
SURFACE RL: 14.0 m
DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered					ML	FILL: Sandy SILT, dark brown, traces of gravel, fine to medium grained, subrounded	D	Fr			FILL/TOPSOIL
							SLATE: Pale grey/white, inferred very low to low strength, foliated, recovered as Silty SAND Traces of pink					HIGHLY TO MODERATELY WEATHERED SLATE
							Hole Terminated at 1.50 m					

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density	V	Very Loose	Density Index <15%	
		HP Hand Penetrometer test (UCS kPa)			L	Loose	Density Index 15 - 35%	
					MD	Medium Dense	Density Index 35 - 65%	
					D	Dense	Density Index 65 - 85%	
					VD	Very Dense	Density Index 85 - 100%	

RG 2.00.3 LIB G.L.B. Log, RG NON-CORED BOREHOLE - TEST PIT, RGS21087.1 BH200 SERIES LOGS.GPJ <<DrawingFile>> 30/1/2024 11:05 10.03.00.09 Dargal Lab and in Situ Tool - DGD Lib, RG 2.00.3 2022-03-03 Pjt, RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH208**

CLIENT: Land Dynamics Australia

PAGE: 1 of 1

PROJECT NAME: Stage 1, The Sanctuary

JOB NO: RGS21087.1

SITE LOCATION: John Oxley Drive, Port Macquarie

LOGGED BY: GC

TEST LOCATION: Lot 116/117

DATE: 20/12/23

DRILL TYPE: RGS Ute Mounted Drill Rig

EASTING: 485414 m

SURFACE RL: 14.0 m

BOREHOLE DIAMETER: 120 mm

INCLINATION: 90°

NORTHING: 6519680 m

DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered					ML	FILL: Sandy SILT, dark brown, sand, fine to medium grained	D	Fr			FILL/TOPSOIL
								M < w _p	Fr / VSt	HP	220	
						CI	Silty CLAY: Medium plasticity, pale red/white, traces of rock fabric			HP	250	EXTREMELY WEATHERED SLATE
		0.70m		13.5			0.40m					
		U50										
		1.00m		13.0						HP	200	
				12.5			1.50m					
				12.0								
				11.5								
							Hole Terminated at 1.50 m					

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density	V	Very Loose	Density Index <15%	
		HP Hand Penetrometer test (UCS kPa)			L	Loose	Density Index 15 - 35%	
					MD	Medium Dense	Density Index 35 - 65%	
					D	Dense	Density Index 65 - 85%	
					VD	Very Dense	Density Index 85 - 100%	

RG 2.00.3 LIB GLB Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH208 SERIES LOGS.GPJ <DrawingFile> 30/1/2024 11:05 10.03.00.09 Dargal Lab and in Situ Tool - DGD Lib RG 2.00.3 2022-03-03 PJT RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH209**CLIENT:** Land Dynamics Australia**PAGE:** 1 of 1**PROJECT NAME:** Stage 1, The Sanctuary**JOB NO:** RGS21087.1**SITE LOCATION:** John Oxley Drive, Port Macquarie**LOGGED BY:** GC**TEST LOCATION:** Lot 118/119**DATE:** 20/12/23**DRILL TYPE:** RGS Ute Mounted Drill Rig**EASTING:** 485374 m**SURFACE RL:** 16.5 m**BOREHOLE DIAMETER:** 120 mm**INCLINATION:** 90°**NORTHING:** 6519671 m**DATUM:** AHD


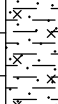

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered					ML	FILL: Sandy SILT, dark brown	D	Fr			FILL/TOPSOIL
							SLATE: Pale grey/white, inferred very low to low strength, foliated, recovered as Sandy GRAVEL					HIGHLY TO MODERATELY WEATHERED SLATE
							Hole Terminated at 1.50 m					

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density	V	Very Loose	Density Index <15%	
		HP Hand Penetrometer test (UCS kPa)			L	Loose	Density Index 15 - 35%	
					MD	Medium Dense	Density Index 35 - 65%	
					D	Dense	Density Index 65 - 85%	
					VD	Very Dense	Density Index 85 - 100%	

BOREHOLE NO: **BH210**




PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

DRILL TYPE:	RGS Ute Mounted Drill Rig	EASTING:	485370 m	SURFACE RL:	16.5 m
BOREHOLE DIAMETER:	120 mm	INCLINATION:	90°	NORTHING:	6519707 m
				DATUM:	AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations	
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result		
AD/T	Not Encountered					ML	FILL: Sandy SILT, dark brown, sand, fine to medium grained Sandy Silty CLAY: Medium to high plasticity, pale red with brown/red mottling, traces of rock fabric	D	Fr	HP	210	FILL/TOPSOIL	
		0.30m				CH		M < w _p	Fr / VSt			EXTREMELY WEATHERED SLATE	
		U50	16.0	0.5								HP	220
		0.70m											
			15.5	1.0									
			15.0	1.5			1.50m						
							Hole Terminated at 1.50 m						
			14.5	2.0									
			14.0	2.5									

LEGEND:

Water

-  Water Level
(Date and time shown)
-  Water Inflow
-  Water Outflow

Strata Changes

- — Gradational or transitional strata
—— Definitive or distinct strata change

Notes, Samples and Tests

- | | |
|-----------------|-----------------------------|
| U ₅₀ | 50mm Diameter tube sample |
| CBR | Bulk sample for CBR testing |
| E | Environmental sample |
| ASS | Acid Sulfate Soil Sample |
| B | Bulk Sample |

Field Tests

- | | |
|----------|---|
| PID | Photoionisation detector reading (ppm) |
| DCP(x-y) | Dynamic penetrometer test (test depth interval shown) |
| HP | Hand Penetrometer test (UCS kPa) |

Consistency	

- | | | |
|-----|------------|-----------|
| VS | Very Soft | <25 |
| S | Soft | 25 - 50 |
| F | Firm | 50 - 100 |
| St | Stiff | 100 - 200 |
| VSt | Very Stiff | 200 - 400 |
| H | Hard | >400 |
| Fr | Frangible | |

[illegible]

- <25
25 - 50
50 - 100
100 - 200
200 - 400
>400

Moisture Condition

- | | |
|-------|---------------|
| D | Dry |
| M | Moist |
| W | Wet |
| W_p | Plastic Limit |
| W_L | Liquid Limit |

Density

- | <u>Density</u> | | | |
|----------------|--------------|-------------------------|--|
| V | Very Loose | Density Index <15% | |
| L | Loose | Density Index 15 - 35% | |
| MD | Medium Dense | Density Index 35 - 65% | |
| D | Dense | Density Index 65 - 85% | |
| VD | Very Dense | Density Index 85 - 100% | |



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH211**

CLIENT: Land Dynamics Australia

PAGE: 1 of 1

PROJECT NAME: Stage 1, The Sanctuary

JOB NO: RGS21087.1

SITE LOCATION: John Oxley Drive, Port Macquarie

LOGGED BY: GC

TEST LOCATION: Lot 122/123

DATE: 20/12/23

DRILL TYPE: RGS Ute Mounted Drill Rig

EASTING: 485394 m

SURFACE RL: 16.3 m

BOREHOLE DIAMETER: 120 mm

INCLINATION: 90°

NORTHING: 6519736 m

DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered		16.0	0.15m		ML	FILL: Sandy SILT, dark brown, traces of gravel, fine to medium grained	D	Fr	HP	250	FILL/TOPSOIL
						CI	Silty CLAY: Medium plasticity, pale red/pale grey, traces of rock fabric	M < w _p	Fr / VSt			EXTREMELY WEATHERED SLATE
							SLATE: Pale orange/pale brown, inferred very low to low strength, recovered as Sandy SILT					HIGHLY TO MODERATELY WEATHERED SLATE
				0.5								
				1.0								
				1.5								
				1.50m			Hole Terminated at 1.50 m					
				14.5								
				2.0								
				14.0								
				2.5								
				13.5								

LEGEND:

Water

- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes

- Gradational or transitional strata
- Definitive or distinct strata change

Notes, Samples and Tests

- U₅₀ 50mm Diameter tube sample
- CBR Bulk sample for CBR testing
- E Environmental sample
- ASS Acid Sulfate Soil Sample
- B Bulk Sample

Field Tests

- PID Photoionisation detector reading (ppm)
- DCP(x-y) Dynamic penetrometer test (test depth interval shown)
- HP Hand Penetrometer test (UCS kPa)

Consistency

- VS Very Soft
- S Soft
- F Firm
- St Stiff
- VSt Very Stiff
- H Hard
- Fb Friable

UCS (kPa)

- <25
- 25 - 50
- 50 - 100
- 100 - 200
- 200 - 400
- >400

Moisture Condition

- D Dry
- M Moist
- W Wet
- W_p Plastic Limit
- W_L Liquid Limit


Density

- V Very Loose
 - L Loose
 - MD Medium Dense
 - D Dense
 - VD Very Dense
- Density Index <15%
Density Index 15 - 35%
Density Index 35 - 65%
Density Index 65 - 85%
Density Index 85 - 100%

BOREHOLE NO: **BH212**




PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

DRILL TYPE:	RGS Ute Mounted Drill Rig	EASTING:	485381 m	SURFACE RL:	15.8 m
BOREHOLE DIAMETER:	120 mm	INCLINATION:	90°	NORTHING:	6519769 m
				DATUM:	AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result			
AD/T	Not Encountered		15.5		ML	0.10m	FILL: Sandy SILT, pale brown	D	Fr	HP	250	FILL/TOPSOIL		
					CH		Sandy CLAY: Medium plasticity, red, sand, fine to medium grained, traces of pale brown/white	M < w _p	Fr / Vst			RESIDUAL		
													HP	300
													HP	300
			14.0	2.0			Hole Terminated at 1.50 m							
			13.5	2.5										
			13.0											

LEGEND:

Water

-  Water Level
 (Date and time shown)
 Water Inflow
 Water Outflow

Strata Changes

- — Gradational or transitional strata
—— Definitive or distinct strata change

Notes, Samples and Tests

- | | |
|-----------------|-----------------------------|
| U ₅₀ | 50mm Diameter tube sample |
| CBR | Bulk sample for CBR testing |
| E | Environmental sample |
| ASS | Acid Sulfate Soil Sample |
| B | Bulk Sample |

Field Tests

- | | |
|----------|---|
| PID | Photoionisation detector reading (ppm) |
| DCP(x-y) | Dynamic penetrometer test (test depth interval shown) |
| HP | Hand Penetrometer test (UCS kPa) |

Consistency

- | | | |
|-----|------------|-----------|
| VS | Very Soft | <25 |
| S | Soft | 25 - 50 |
| F | Firm | 50 - 100 |
| St | Stiff | 100 - 200 |
| VSt | Very Stiff | 200 - 400 |
| H | Hard | >400 |
| Fb | Frangible | |

UCS (kPa)

- <25
25 - 50
50 - 100
100 - 200
200 - 400
>400

Moisture Condition

- | | |
|-------|---------------|
| D | Dry |
| M | Moist |
| W | Wet |
| W_p | Plastic Limit |
| W_L | Liquid Limit |

Density

- | Density | | | |
|---------|--------------|-------------------------|--|
| V | Very Loose | Density Index <15% | |
| L | Loose | Density Index 15 - 35% | |
| MD | Medium Dense | Density Index 35 - 65% | |
| D | Dense | Density Index 65 - 85% | |
| VD | Very Dense | Density Index 85 - 100% | |



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH213**CLIENT:** Land Dynamics Australia**PAGE:** 1 of 1**PROJECT NAME:** Stage 1, The Sanctuary**JOB NO:** RGS21087.1**SITE LOCATION:** John Oxley Drive, Port Macquarie**LOGGED BY:** GC**TEST LOCATION:** Lot 126/127**DATE:** 20/12/23**DRILL TYPE:** RGS Ute Mounted Drill Rig**EASTING:** 485358 m**SURFACE RL:** 16.0 m**BOREHOLE DIAMETER:** 120 mm**INCLINATION:** 90°**NORTHING:** 6519786 m**DATUM:** AHD







Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered					CL	FILL: Sandy CLAY, low plasticity, dark brown	M < w _p	Fr	HP	450	FILL/TOPSOIL
						CH	Sandy CLAY: Medium to high plasticity, red, sand, fine to medium grained					RESIDUAL
			15.5	0.5								
			15.0	1.0								
			14.5	1.5						HP	450	
							Hole Terminated at 1.50 m					
			14.0	2.0								
			13.5	2.5								

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density		V	Very Loose	Density Index <15%
		HP Hand Penetrometer test (UCS kPa)		L		L	Loose	Density Index 15 - 35%
				MD		MD	Medium Dense	Density Index 35 - 65%
				D		D	Dense	Density Index 65 - 85%
				VD		VD	Very Dense	Density Index 85 - 100%

BOREHOLE NO: **BH214**

PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

DRILL TYPE:	RGS Ute Mounted Drill Rig	EASTING:	485340 m	SURFACE RL:	16.8 m
BOREHOLE DIAMETER:	120 mm	INCLINATION:	90°	NORTHING:	6519754 m
				DATUM:	AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations	
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result		
AD/T	Not Encountered					CL	FILL: Sandy CLAY, low plasticity, dark brown	M < w _p	Fr			FILL/TOPSOIL	
		0.30m	16.5		CH	Sandy CLAY: Medium to high plasticity, yellow/pale brown, sand, fine to medium grained, traces of rock fabric	Fr / VSt		HP	300	EXTREMELY WEATHERED DOLERITE		
		U50	0.5				HP		250				
			0.70m	16.0					HP	230			
				1.5		1.50m	Hole Terminated at 1.50 m						
				15.0									
				2.0									
				14.5									
				2.5									
				14.0									
LEGEND:			Notes, Samples and Tests					Consistency		UCS (kPa)		Moisture Condition	
Water								VS Very Soft		<25		D Dry	
 Water Level (Date and time shown)			U ₅₀ 50mm Diameter tube sample					S Soft		25 - 50		M Moist	
 Water Inflow			CBR Bulk sample for CBR testing					F Firm		50 - 100		W Wet	
 Water Outflow			E Environmental sample					St Stiff		100 - 200		W _p Plastic Limit	
Strata Changes			ASS Acid Sulfate Soil Sample					VSt Very Stiff		200 - 400		W _L Liquid Limit	
			B Bulk Sample					H Hard		>400			
								Fb Friable					
								Density		V Very Loose		Density Index <15%	
 Gradational or transitional strata			PID Photoionisation detector reading (ppm)					L Loose				Density Index 15 - 35%	
 Definitive or distinct strata change			DCP(x-y) Dynamic penetrometer test (test depth interval shown)					MD Medium Dense				Density Index 35 - 65%	
			HP Hand Penetrometer test (UCS kPa)					D Dense				Density Index 65 - 85%	
								VD Very Dense				Density Index 85 - 100%	



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH215**

CLIENT: Land Dynamics Australia

PAGE: 1 of 1

PROJECT NAME: Stage 1, The Sanctuary

JOB NO: RGS21087.1

SITE LOCATION: John Oxley Drive, Port Macquarie

LOGGED BY: GC

TEST LOCATION: Lot 130/131

DATE: 20/12/23

DRILL TYPE: RGS Ute Mounted Drill Rig

EASTING: 485345 m

SURFACE RL: 16.8 m

BOREHOLE DIAMETER: 120 mm

INCLINATION: 90°

NORTHING: 6519726 m

DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations	
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result		
AD/T	Not Encountered		16.5	0.5		ML	FILL: Sandy SILT, dark brown, traces of gravel, fine grained, subangular	D	Fr	HP	210	FILL/TOPSOIL	
						CH	Sandy CLAY: Medium plasticity, pale brown/yellow, traces of rock fabric	M < w _p	Fr / VSt			EXTREMELY WEATHERED DOLERITE	
			16.0	1.0						HP	200		
			15.5	1.5						HP	220		
			15.0	2.0			Hole Terminated at 1.50 m						
			14.5	2.5									
			14.0										
LEGEND:					Notes, Samples and Tests					Consistency		UCS (kPa)	Moisture Condition
Water					U ₅₀ 50mm Diameter tube sample CBR Bulk sample for CBR testing E Environmental sample ASS Acid Sulfate Soil Sample B Bulk Sample					VS Very Soft S Soft F Firm St Stiff VSt Very Stiff H Hard Fb Friable		<25 25 - 50 50 - 100 100 - 200 200 - 400 >400	D Dry M Moist W Wet W _p Plastic Limit W _L Liquid Limit
Strata Changes					Field Tests PID Photoionisation detector reading (ppm) DCP(x-y) Dynamic penetrometer test (test depth interval shown) HP Hand Penetrometer test (UCS kPa)					Density V Very Loose L Loose MD Medium Dense D Dense VD Very Dense			Density Index <15% Density Index 15 - 35% Density Index 35 - 65% Density Index 65 - 85% Density Index 85 - 100%

RG 2.00.3 LIB GLB Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH200 SERIES LOGS.GPJ <<DrawingFile>> 30/1/2024 11:05 10.03.00.09 Dargal Lab and in Situ Tool - DGD Lib RG 2.00.3 2022-03-03 PJT RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH216**
PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

CLIENT: Land Dynamics Australia
PROJECT NAME: Stage 1, The Sanctuary
SITE LOCATION: John Oxley Drive, Port Macquarie
TEST LOCATION: Lot 132/133

DRILL TYPE: RGS Ute Mounted Drill Rig
BOREHOLE DIAMETER: 120 mm
EASTING: 485341 m
NORTHING: 6519697 m
SURFACE RL: 17.3 m
DATUM: AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered		17.0	0.20m		ML	FILL: Sandy SILT, dark brown, traces of gravel, fine grained, subangular	D	Fr			FILL/TOPSOIL
							SLATE: Yellow/pale grey, very low to low strength, recovered as Sandy SILT					HIGHLY TO MODERATELY WEATHERED SLATE
				0.5								
				1.0								
				1.5								
				1.50m			Hole Terminated at 1.50 m					
				2.0								
				2.5								
				3.0								
				3.5								
				4.0								
				4.5								
				5.0								
				5.5								
				6.0								
				6.5								
				7.0								
				7.5								
				8.0								
				8.5								
				9.0								
				9.5								
				10.0								
				10.5								
				11.0								
				11.5								
				12.0								
				12.5								
				13.0								
				13.5								
				14.0								
				14.5								
				15.0								
				15.5								
				16.0								
				16.5								
				17.0								





LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density		V	Very Loose	Density Index <15%
		HP Hand Penetrometer test (UCS kPa)		L	Loose			Density Index 15 - 35%
				MD	Medium Dense			Density Index 35 - 65%
				D	Dense			Density Index 65 - 85%
				VD	Very Dense			Density Index 85 - 100%

RG 2.00.3 LIB GLB Log RG NON-CORED BOREHOLE - TEST PIT RGS21087.1 BH200 SERIES LOGS.GPJ <DrawingFile> 30/1/2024 11:08 10.03.00.08 Dargal Lab and in Situ Tool - DGD Lib RG 2.00.3 2022-03-03 Pjt RG 2.00.0 2021-06-30



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH217**CLIENT:** Land Dynamics Australia**PAGE:** 1 of 1**PROJECT NAME:** Stage 1, The Sanctuary**JOB NO:** RGS21087.1**SITE LOCATION:** John Oxley Drive, Port Macquarie**LOGGED BY:** GC**TEST LOCATION:** Lot 108/109**DATE:** 20/12/23**DRILL TYPE:** RGS Ute Mounted Drill Rig**EASTING:** 485464 m**SURFACE RL:** 13.8 m**BOREHOLE DIAMETER:** 120 mm**INCLINATION:** 90°**NORTHING:** 6519664 m**DATUM:** AHD

Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations	
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result		
AD/T	Not Encountered		13.5		ML	0.15m	FILL: Sandy SILT, dark brown, traces of gravel, fine to medium grained, subangular	D	Fr	HP	220	FILL/TOPSOIL	
					CI	FILL: Sandy Silty CLAY, medium plasticity, pale red/pale grey, some gravel, fine to medium grained, subangular	M < w _p	Fr / VSt	FILL-CLAY				
						0.5							
						13.0							
				1.0						HP	250		
				12.5									
				1.5		1.50m				HP	230		
				12.0			Hole Terminated at 1.50 m						
				2.0									
				11.5									
				2.5									
				11.0									
LEGEND: Water  Water Level (Date and time shown)  Water Inflow  Water Outflow Strata Changes --- Gradational or transitional strata — Definitive or distinct strata change					Notes, Samples and Tests U ₅₀ 50mm Diameter tube sample CBR Bulk sample for CBR testing E Environmental sample ASS Acid Sulfate Soil Sample B Bulk Sample Field Tests PID Photoionisation detector reading (ppm) DCP(x-y) Dynamic penetrometer test (test depth interval shown) HP Hand Penetrometer test (UCS kPa)					Consistency VS Very Soft <25 S Soft 25 - 50 F Firm 50 - 100 St Stiff 100 - 200 VSt Very Stiff 200 - 400 H Hard >400 Fb Friable		UCS (kPa) <25 25 - 50 50 - 100 100 - 200 200 - 400 >400	Moisture Condition D Dry M Moist W Wet W _p Plastic Limit W _L Liquid Limit
										Density V Very Loose L Loose MD Medium Dense D Dense VD Very Dense		Density Index <15% Density Index 15 - 35% Density Index 35 - 65% Density Index 65 - 85% Density Index 85 - 100%	



ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH218**

CLIENT: Land Dynamics Australia
PROJECT NAME: Stage 1, The Sanctuary
SITE LOCATION: John Oxley Drive, Port Macquarie
TEST LOCATION: Lot 101/102

PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

DRILL TYPE: RGS Ute Mounted Drill Rig
BOREHOLE DIAMETER: 120 mm
INCLINATION: 90°
EASTING: 485494 m
NORTHING: 6519666 m
SURFACE RL: 11.5 m
DATUM: AHD




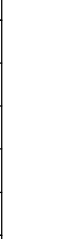
Drilling and Sampling					Material description and profile information					Field Test		Structure and additional observations
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	
AD/T	Not Encountered		11.0	0.5		ML	FILL: Sandy SILT, dark brown, traces of gravel, fine to medium grained, subrounded	D	Fr	HP	350	FILL/TOPSOIL
						CI	FILL: Silty CLAY, low to medium plasticity, pale grey/white with red mottling	M < w _p	Fr / VSt			FILL-CLAY
							SLATE: Pale grey/pale red, inferred very low to low strength, recovered as Sandy SILT					HIGHLY WEATHERED SLATE
			10.0	1.5			Hole Terminated at 1.50 m					
			9.5	2.0								
			9.0	2.5								

LEGEND:		Notes, Samples and Tests		Consistency		UCS (kPa)	Moisture Condition	
Water		U ₅₀ 50mm Diameter tube sample		VS	Very Soft	<25	D	Dry
Water Level (Date and time shown)		CBR Bulk sample for CBR testing		S	Soft	25 - 50	M	Moist
Water Inflow		E Environmental sample		F	Firm	50 - 100	W	Wet
Water Outflow		ASS Acid Sulfate Soil Sample		St	Stiff	100 - 200	W _p	Plastic Limit
Strata Changes		B Bulk Sample		VSt	Very Stiff	200 - 400	W _L	Liquid Limit
Gradational or transitional strata		Field Tests		H	Hard	>400		
Definitive or distinct strata change		PID Photoionisation detector reading (ppm)		Fb	Friable			
		DCP(x-y) Dynamic penetrometer test (test depth interval shown)		Density	V	Very Loose	Density Index <15%	
		HP Hand Penetrometer test (UCS kPa)			L	Loose	Density Index 15 - 35%	
					MD	Medium Dense	Density Index 35 - 65%	
					D	Dense	Density Index 65 - 85%	
					VD	Very Dense	Density Index 85 - 100%	

BOREHOLE NO: **BH219**




PAGE: 1 of 1
JOB NO: RGS21087.1
LOGGED BY: GC
DATE: 20/12/23

DRILL TYPE:	RGS Ute Mounted Drill Rig	EASTING:	485454 m	SURFACE RL:	12.5 m
BOREHOLE DIAMETER:	120 mm	INCLINATION:	90°	NORTHING:	6519704 m
				DATUM:	AHD

Drilling and Sampling					Material description and profile information						Field Test		Structure and additional observations	
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics, colour, minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result			
AD/T	Not Encountered		12.0	0.5		ML	FILL: Sandy SILT, dark brown, traces of gravel, fine to medium grained, subangular, sand, fine to medium grained	D	Fr	HP	390	FILL/TOPSOIL		
						CI	0.15m	Sandy CLAY: Medium plasticity, red, sand, fine to medium grained	M < w _p			Fr / VSt	RESIDUAL	
													HP	380
													HP	250
			11.5	1.0			Some pale brown mottling							
			11.0	1.5								1.50m		
			10.5	2.0			Hole Terminated at 1.50 m							
			10.0	2.5										

LEGEND:

Water

-  Water Level
 (Date and time shown)
 Water Inflow
 Water Outflow

Strata Changes

- — Gradational or transitional strata
—— Definitive or distinct strata change

Notes, Samples and Tests

- | | |
|-----------------|-----------------------------|
| U ₅₀ | 50mm Diameter tube sample |
| CBR | Bulk sample for CBR testing |
| E | Environmental sample |
| ASS | Acid Sulfate Soil Sample |
| B | Bulk Sample |

Field Tests

- | | |
|----------|---|
| PID | Photoionisation detector reading (ppm) |
| DCP(x-y) | Dynamic penetrometer test (test depth interval shown) |
| HP | Hand Penetrometer test (UCS kPa) |

Consistency

- | | | |
|-----|------------|-----------|
| VS | Very Soft | <25 |
| S | Soft | 25 - 50 |
| F | Firm | 50 - 100 |
| St | Stiff | 100 - 200 |
| VSt | Very Stiff | 200 - 400 |
| H | Hard | >400 |
| Fb | Frangible | |

UCS (kPa)
100
200
300
400
500
600
700
800
900
1000
1100
1200
1300
1400
1500
1600
1700
1800
1900
2000
2100
2200
2300
2400
2500
2600
2700
2800
2900
3000
3100
3200
3300
3400
3500
3600
3700
3800
3900
4000
4100
4200
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8000
8100
8200
8300
8400
8500
8600
8700
8800
8900
9000
9100
9200
9300
9400
9500
9600
9700
9800
9900
10000

- <25
25 - 50
50 - 100
100 - 200
200 - 400
>400

Moisture Condition

- | | |
|-------|---------------|
| D | Dry |
| M | Moist |
| W | Wet |
| W_p | Plastic Limit |
| W_L | Liquid Limit |

Density

- | Density | | | |
|---------|--------------|-------------------------|--|
| V | Very Loose | Density Index <15% | |
| L | Loose | Density Index 15 - 35% | |
| MD | Medium Dense | Density Index 35 - 65% | |
| D | Dense | Density Index 65 - 85% | |
| VD | Very Dense | Density Index 85 - 100% | |



Appendix B

Laboratory Test Result Sheets

Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583A
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH202 - (0.4 - 0.7m)
Material: Clay
Material Source: On-Site Insitu



Newcastle Laboratory
2 Murray Dwyer Circuit Mayfield West NSW 2304
Phone: (02) 4968 4468

Email: brentcullen@qualtest.com.au

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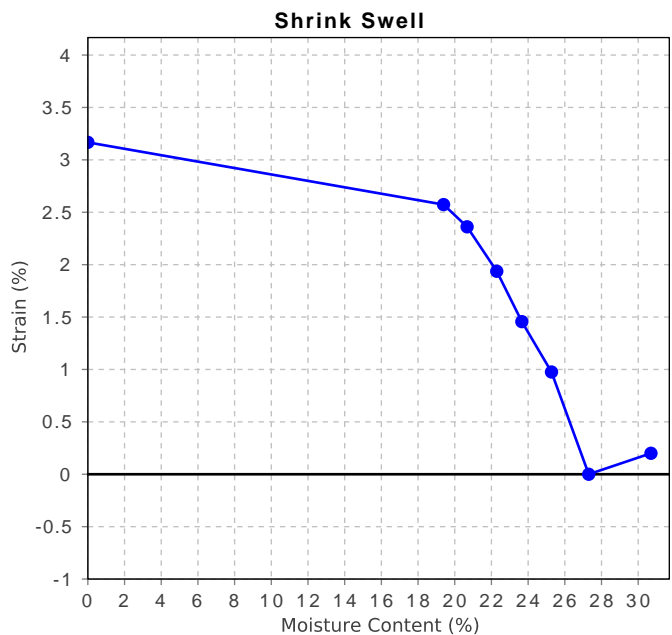
Approved Signatory: Brent Cullen

Engineering Geologist

NATA Accredited Laboratory Number: 18686

B. Cullen

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	1.8
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	3.2
Estimated % by volume of significant inert inclusions	6
Cracking	Moderately Cracked
Crumbling	No
Moisture Content (%)	27.3
Swell Test	
Initial Pocket Penetrometer (kPa)	500
Final Pocket Penetrometer (kPa)	430
Initial Moisture Content (%)	27.7
Final Moisture Content (%)	30.7
Swell (%)	-0.2
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583B
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH203 - (0.3 - 0.7m)
Material: Clay
Material Source: On-Site Insitu



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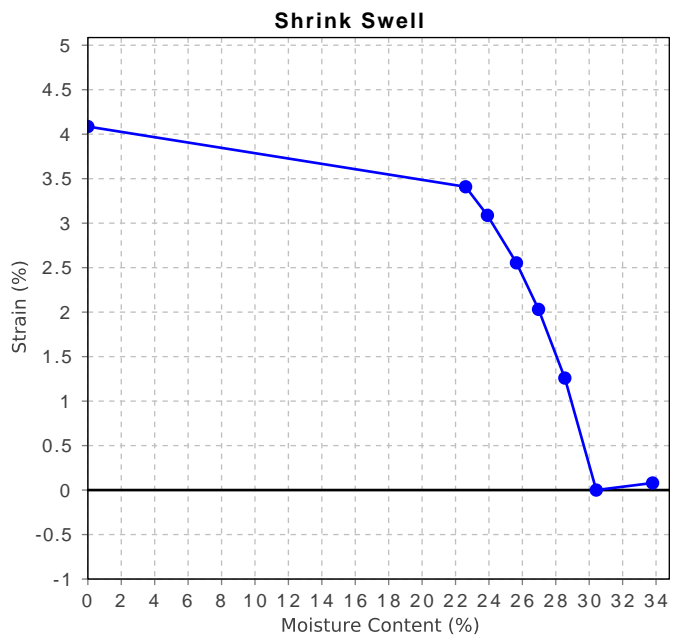
Approved Signatory: Brent Cullen

Engineering Geologist

NATA Accredited Laboratory Number: 18686

B. Cullen

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	2.3
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	4.1
Estimated % by volume of significant inert inclusions	6
Cracking	Slightly Cracked
Crumbling	No
Moisture Content (%)	30.4
Swell Test	
Initial Pocket Penetrometer (kPa)	480
Final Pocket Penetrometer (kPa)	450
Initial Moisture Content (%)	29.4
Final Moisture Content (%)	33.8
Swell (%)	-0.1
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583C
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH205 - (0.4 - 0.7m)
Material: Clay
Material Source: On-Site Insitu



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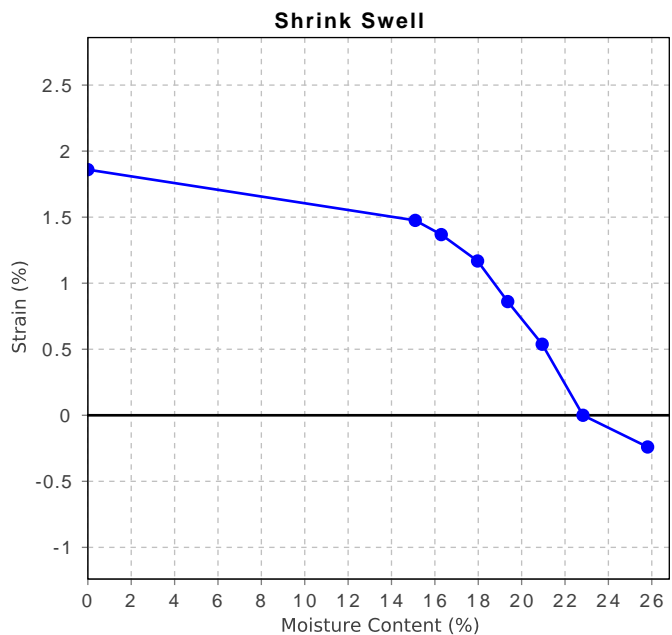


Approved Signatory: Brent Cullen

Engineering Geologist

NATA Accredited Laboratory Number: 18686

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	1.1
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	1.9
Estimated % by volume of significant inert inclusions	4
Cracking	Slightly Cracked
Crumbling	No
Moisture Content (%)	22.8
Swell Test	
Initial Pocket Penetrometer (kPa)	>600
Final Pocket Penetrometer (kPa)	520
Initial Moisture Content (%)	22.8
Final Moisture Content (%)	25.8
Swell (%)	0.2
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583D
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH208 - (0.7 - 1.1m)
Material: Clay
Material Source: On-Site Insitu



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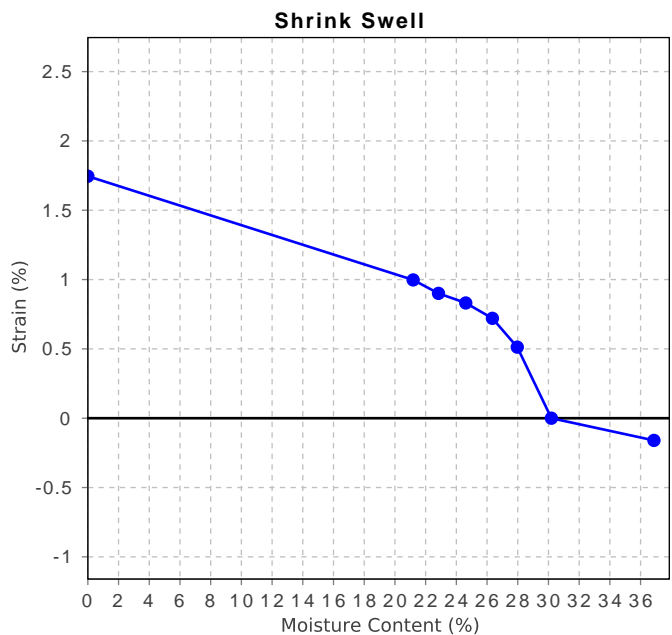
Approved Signatory: Brent Cullen

Engineering Geologist

NATA Accredited Laboratory Number: 18686

B. Cullen

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	1.0
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	1.7
Estimated % by volume of significant inert inclusions	6
Cracking	Moderately Cracked
Crumbling	No
Moisture Content (%)	30.2
Swell Test	
Initial Pocket Penetrometer (kPa)	>600
Final Pocket Penetrometer (kPa)	420
Initial Moisture Content (%)	29.2
Final Moisture Content (%)	36.9
Swell (%)	0.2
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583E
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH210 - (0.3 - 0.7m)
Material: Clay
Material Source: On-Site Insitu



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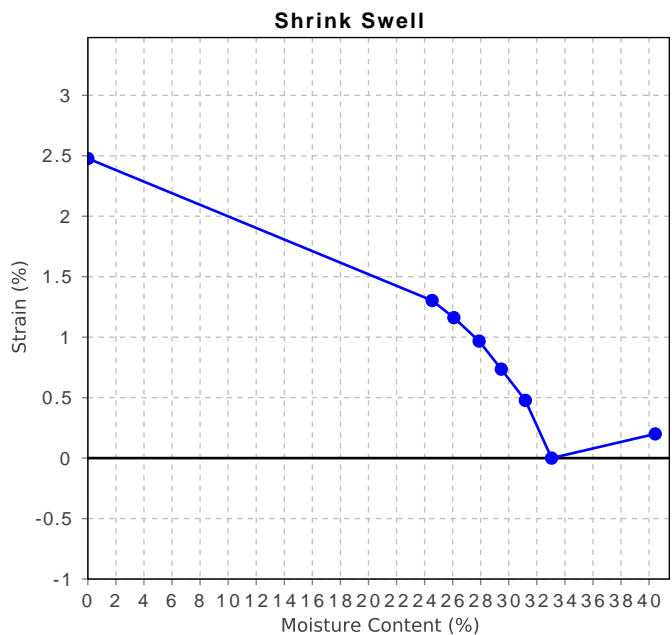
Approved Signatory: Brent Cullen

Engineering Geologist

NATA Accredited Laboratory Number: 18686

B. Cullen

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	1.4
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	2.5
Estimated % by volume of significant inert inclusions	3
Cracking	Moderately Cracked
Crumbling	No
Moisture Content (%)	33.1
Swell Test	
Initial Pocket Penetrometer (kPa)	350
Final Pocket Penetrometer (kPa)	330
Initial Moisture Content (%)	32.7
Final Moisture Content (%)	40.4
Swell (%)	-0.2
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583F
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH212 - (0.3 - 0.7m)
Material: Clay
Material Source: On-Site Insitu



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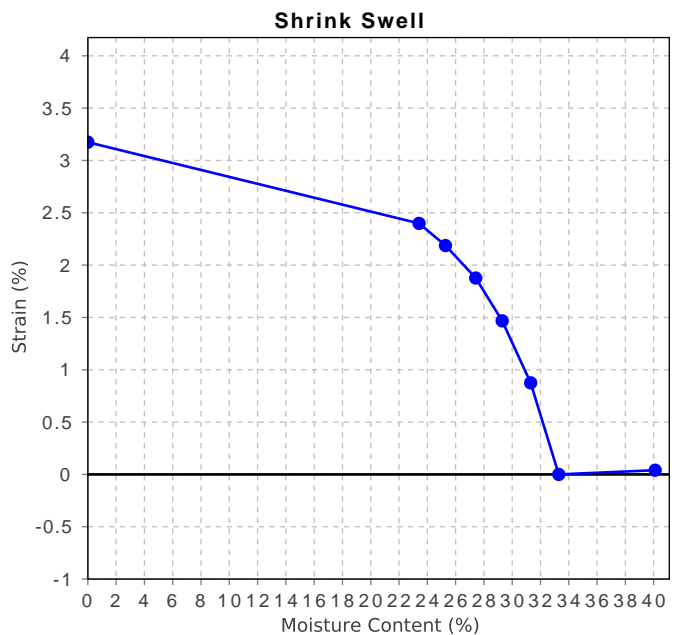
Approved Signatory: Brent Cullen

Engineering Geologist

NATA Accredited Laboratory Number: 18686

B. Cullen

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	1.8
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	3.2
Estimated % by volume of significant inert inclusions	3
Cracking	Slightly Cracked
Crumbling	No
Moisture Content (%)	33.3
Swell Test	
Initial Pocket Penetrometer (kPa)	>600
Final Pocket Penetrometer (kPa)	520
Initial Moisture Content (%)	34.1
Final Moisture Content (%)	40.1
Swell (%)	-0.0
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583G
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH214 - (0.3 - 0.7m)
Material: Clay
Material Source: On-Site Insitu



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Engineering Geologist

NATA Accredited Laboratory Number: 18686

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Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	1.0
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	1.7
Estimated % by volume of significant inert inclusions	2
Cracking	Moderately Cracked
Crumbling	No
Moisture Content (%)	27.9
Swell Test	
Initial Pocket Penetrometer (kPa)	>600
Final Pocket Penetrometer (kPa)	510
Initial Moisture Content (%)	26.8
Final Moisture Content (%)	33.9
Swell (%)	0.2
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583H
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 09/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH217 - (0.3 - 0.7m)
Material: Clay
Material Source: On-Site Insitu



Newcastle Laboratory
2 Murray Dwyer Circuit Mayfield West NSW 2304
Phone: (02) 4968 4468

Email: brentcullen@qualtest.com.au

Accredited for compliance with ISO/IEC 17025 - Testing

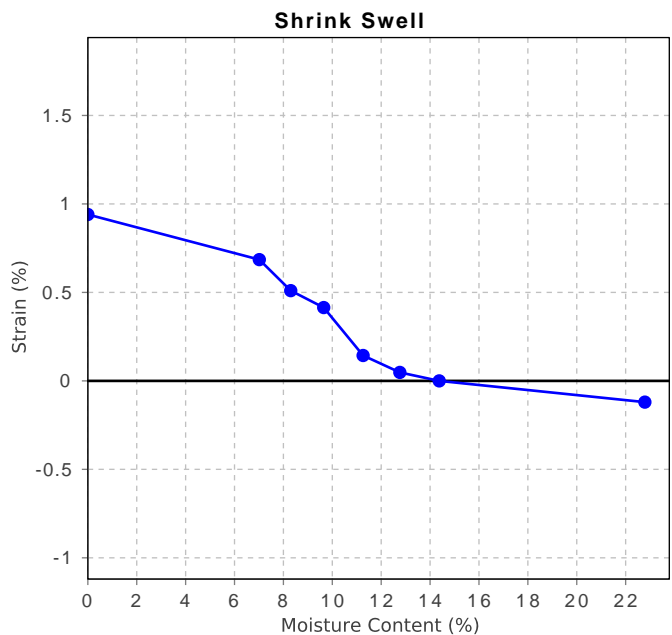


Approved Signatory: Brent Cullen

Engineering Geologist

NATA Accredited Laboratory Number: 18686

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)	
Iss (%)	0.6
Visual Description	Clay
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.	
Core Shrinkage Test	
Shrinkage Strain - Oven Dried (%)	0.9
Estimated % by volume of significant inert inclusions	7
Cracking	Slightly Cracked
Crumbling	No
Moisture Content (%)	14.4
Swell Test	
Initial Pocket Penetrometer (kPa)	>600
Final Pocket Penetrometer (kPa)	370
Initial Moisture Content (%)	13.7
Final Moisture Content (%)	22.8
Swell (%)	0.1
* NATA Accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: MNC16P-0001-29
Issue Number: 1
Date Issued: 16/01/2024
Client: Regional Geotechnical Solutions Pty Ltd
44 Bent Street, Wingham NSW 2429
Project Number: MNC16P-0001
Project Name: Various Testing
Project Location: The Sanctuary John Oxley Drive, Port Macquarie, NSW
Client Reference: RGS21087.1
Work Request: 1583
Sample Number: NEW23S-1583I
Date Sampled: 20/12/2023
Dates Tested: 21/12/2023 - 15/01/2024
Sampling Method: Sampled by Client
The results apply to the sample as received
Sample Location: BH218 - (0.3 - 0.6m)
Material: Clay
Material Source: On-Site Insitu



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Engineering Geologist

NATA Accredited Laboratory Number: 18686

Atterberg Limit (AS1289 3.1.2 & 3.2.1 & 3.3.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	41		
Plastic Limit (%)	28		
Plasticity Index (%)	13		
Linear Shrinkage (AS1289 3.4.1)		Min	Max
Moisture Condition Determined By	AS 1289.3.1.1		
Linear Shrinkage (%)	5.0		
Cracking Crumbling Curling	Cracking		